

**Ministry of Higher Education
and Scientific Research
University of Diyala
College of Engineering**



Structural Behavior of Reinforced Self- Compacted Concrete Box Beams

**A Thesis Submitted to the Council of College of Engineering,
University of Diyala in Partial Fulfillment of the
Requirements for the Degree of Master of Science in Civil
Engineering**

By

Zahraa Sermed Zuhdiy

(B.Sc. in Civil Engineering, 2017)

Supervised by

Prof. Dr. Ali Laftah Abbas

May, 2021

IRAQ

Ramadan, 1442

بِسْمِ اللَّهِ الرَّحْمَنِ الرَّحِيمِ

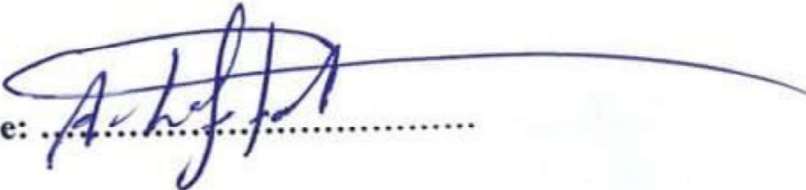
﴿ وَقَدْ رَبُّ زَيْنِ عِمَّا ﴾

صدق الله العظيم

[طه: ١١٤]

Supervisor Certification

I certify that the thesis entitled "**Structural Behavior of Reinforced Self-Compacted Concrete Box Beams**" presented by (Zahraa Sermed Zuhdiy) was prepared under my supervision in Civil Engineering Department, University of Diyala, in partial Fulfillment of the Requirements of the Degree of Master of Science in Civil Engineering.

Signature: 

Supervisor: Prof. Dr. Ali Laftah Abbas

Date: 23/5 / 2021

COMMITTEE DECISION

We certify that we have read the thesis entitled "Structural Behavior of Reinforced Self-Compacted Concrete Box Beams" and we have examined the student (Zahraa Sermed Zuhdiy) in its content and what is related with it, and in our opinion, it is adequate as a thesis for the Degree of Master of Science in Civil Engineering.

Examination Committee

Signature

1- Prof. Dr. Ali Laftah Abbas (Supervisor)



2- Assist.Prof. Qusay W. Ahmed (Member)



3- Lec. Dr. Yahyia Majeed Hameed (Member)



4- Assist.Prof. Dr. Husain Khalaf Jarallah (Chairman)



Prof. Dr. Khattab S. Abdul-Razzaq..... (Head of Department)



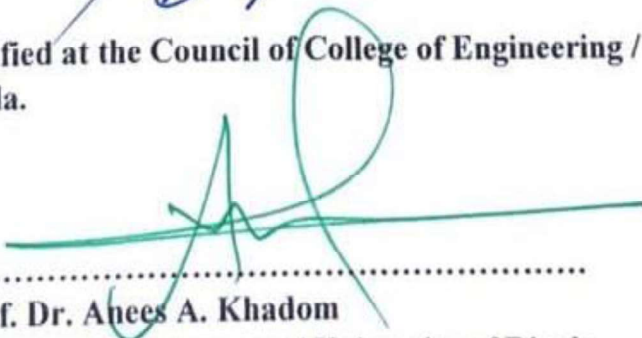
The thesis was ratified at the Council of College of Engineering / University of Diyala.

Signature

Name: Prof. Dr. Anees A. Khadom

Dean of College of Engineering / University of Diyala

Date: / / 2021



DEDICATION



To my parents

with love and gratitude



Zahraa Sermed Zuhdiy

2021

ACKNOWLEDGEMENTS

In the name of Allah, the most gracious, the most merciful. Before anything, I thank **ALLAH** who enabled me to achieve this study.

First, I would like to express my respect, gratitude and appreciation to my supervisor **Prof. Dr. Ali Laftah Abbas** for his encouragement, support and valuable remarks and for guiding me in each step through this work, I'm very proud to be his student.

Great thanks to my teachers in the **Department of Civil Engineering** and the **Structural Engineering Laboratory** staff and for everyone who supported me through this work.

Zahraa Sermed Zuhdiy
2021

Structural Behavior of Reinforced Self-Compacted Concrete Box Beams

By

Zahraa Sermed Zuhdiy

Supervisor

Prof. Dr. Ali Laftah Abbas

Abstract

Box beams are very commonly used structural members due to their high performance, their ability to minimize the dead load and the number of supports which lead to reduce cost. Accordingly, it became a major requirement to focus on studying the structural behavior of this type of structural members.

In this thesis, an experimental study was conducted in order to investigate and improve the structural behavior of reinforced self-compacted concrete box beams by using different number of cells with the same cross-section dimensions as well as studying the effect of using longitudinal shear steel plates that contain vertical and inclined rectangular spacings as shear reinforcement instead of using traditional reinforcement bars (stirrups). Also, the study focused on the effect of using vertical and horizontal corrugated steel plates in strengthening the cells and studying the effect of using circular-shape cell instead of rectangular-shape one with the same web width and strengthening the circular cell with steel plate in the shape of steel pipe.

The experimental program consisted of casting and testing ten reinforced self-compacted concrete box beam specimens with identical cross-section of overall depth of (320mm), top flange width of (420mm), bottom flange width of (270mm), web width of (60mm) and overall length of (1500mm). The specimens were divided into five groups according to the number of cells, reinforcement type, strengthening and the shape of cell. All the specimens

were tested under four-point monotonic-static load to obtain the ultimate load capacity, crack pattern, mode of failure, crack width, central vertical deflection, ductility and strain in concrete and steel.

The test results showed that increasing the number of cells from one cell into two and four cells has increased the ultimate load capacity by (20.12% and 23.37%) respectively and has increased the ultimate deflection by (30.65% and 13.82%) respectively.

Also, the use of longitudinal shear steel plates that contain vertical and inclined rectangular spacings instead of vertical stirrups has increased the ultimate load capacity by (7.14% and 20.12%) respectively and has increased the ultimate deflection by (20.73% and 38.23%) respectively.

It was found that using corrugated steel plates strengthening with vertical and horizontal corrugation has increased the ultimate load by (7.14% and 11.03%) respectively and has increased the ultimate deflection by (10.92% and 2.67%) respectively.

Also, the use of circular cell without and with steel strengthening instead of rectangular cell has increased the ultimate load capacity by (17.85% and 29.22%) respectively and has increased the ultimate deflection by (63.54% and 33.77%) respectively.

Finally, the use of horizontally corrugated steel plate strengthening and circular cell with steel plate strengthening has increased the ultimate load by (3.63% and 20.60%) respectively, with a decrement in the ultimate deflection for the use of horizontally corrugated steel plate strengthening by (7.43%) and an increment in the ultimate deflection by (20.60%) for the use of circular cell with steel plate strengthening compared with the use of vertical corrugated steel plate strengthening.

TABLE OF CONTENTS

No.	Heading	Page No.
	Dedication	I
	Acknowledgements	II
	Abstract	III
	Table of Contents	V
	List of Figures	VIII
	List of Plates	X
	List of Tables	XI
	List of Symbols	XII
	List of Terminologies	XIII
CHAPTER ONE INTRODUCTION		
1.1	General	1
1.1.1	Advantages of Box Girder	2
1.1.2	Disadvantages of Box Girder	3
1.1.3	Multi-Cell Box Girder	3
1.2	Self-Compacted Concrete	4
1.3	Steel Plates	5
1.4	Objective of Study	5
1.5	Study Justification	5
1.6	Research Methodology	6
1.7	Thesis Layout	6
CHAPTER TWO LITERATURE REVIEW		
2.1	General	7
2.2	General Previous Studies	7
2.3	Previous Studies on Multi-Cell Box Girder	17
2.4	Previous Studies on Corrugated Steel Plates	20
2.5	Previous Studies on Shear Steel Plates	24
2.6	Summary and Concluding Remarks	27
2.7	Differences between the Current Study and the Previous Studies	27
CHAPTER THREE EXPERIMENTAL WORK		
3.1	General	28
3.2	Experimental program	28
3.3	Materials	37
3.3.1	Cement	37
3.3.2	Fine Aggregate	39
3.3.3	Coarse Aggregate	40
3.3.4	Limestone Powder	41
3.3.5	High Performance Superplasticizer Concrete Admixture (HPSCA)	41
3.3.6	Water	42
3.3.7	Steel Reinforcement	42
3.3.8	Steel Plates	43
3.4	Molds	45
3.5	Cells	46
3.6	Self-Compacted Concrete Mix Design	46

3.6.1	Mixing procedure	47
3.7	Testing of Fresh SCC	48
3.7.1	Slump Flow and T50 Tests	48
3.7.2	L-box Test	49
3.8	Casting and curing	50
3.9	Properties of Hardened Concrete	52
3.9.1	Compressive Strength of Cube and Cylinder (f_{cu}) & (f_c')	52
3.9.2	Splitting Tensile Strength (f_{ct})	53
3.9.3	Modulus of Rupture Test (f_r)	54
3.9.4	Modulus of Elasticity	55
3.10	Test Measurements and Instrumentation	57
3.10.1	Deflection Measurement	57
3.10.2	Crack Width Measurement	57
3.10.3	Strain Measurements of Steel and Concrete	58
3.10.3.1	Strain Gauge Locations	58
3.10.3.2	Strain Gauge Installation	60
3.10.4	TDS-530 Data Logger	60
3.11	Testing Procedure	61
CHAPTER FOUR RESULTS AND DISCUSSION		
4.1	General	62
4.2	Group A	62
4.2.1	The Ultimate Load of the Tested Box Beams in Group A	63
4.2.2	Crack Pattern, Crack Width and Mode of Failure of the Tested Box Beams in Group A	63
4.2.3	The Load-Deflection of the Tested Box Beams in Group A	66
4.2.4	The Ductility of the Tested Box Beams in Group A	67
4.2.4.1	Deflection Ductility	68
4.2.4.2	Curvature Ductility	68
4.2.5	The Strain of the Tested Box Beams in Group A	68
4.2.5.1	Average Strain in Compression Face of Concrete	70
4.2.5.2	Average Strain in Longitudinal Reinforcement	71
4.2.5.3	Average Strain in Shear Reinforcement	72
4.3	Group B	73
4.3.1	The Ultimate Load of the Tested Box Beams in Group B	74
4.3.2	Crack Pattern, Crack Width and Mode of Failure of the Tested Box Beams in Group B	75
4.3.3	The Load-Deflection of the Tested Box Beams in Group B	77
4.3.4	The Ductility of the Tested Box Beams in Group B	78
4.3.4.1	Deflection Ductility	78
4.3.4.2	Curvature Ductility	79
4.3.5	The Strain of the Tested Box Beams in Group B	79
4.3.5.1	Average Strain in Compression Face of Concrete	81
4.3.5.2	Average Strain in Longitudinal Reinforcement	82
4.3.5.3	Average Strain in Shear Reinforcement	83
4.4	Group C	84
4.4.1	The Ultimate Load of the Tested Box Beams in Group C	85
4.4.2	Crack Pattern, Crack Width and Mode of Failure of the Tested Box Beams in Group C	85
4.4.3	The Load-Deflection of the Tested Box Beams in Group C	87
4.4.4	The Ductility of the Tested Box Beams in Group C	88
4.4.4.1	Deflection Ductility	88

4.4.4.2	Curvature Ductility	89
4.4.5	The Strain of the Tested Box Beams in Group C	89
4.4.5.1	Average Strain in Compression Face of Concrete	91
4.4.5.2	Average Strain in Longitudinal Reinforcement	91
4.4.5.3	Average Strain in Shear Reinforcement	92
4.5	Group D	93
4.5.1	The Ultimate Load of the Tested Box Beams in Group D	94
4.5.2	Crack Pattern, Crack Width and Mode of Failure of the Tested Box Beams in Group D	94
4.5.3	The Load-Deflection of the Tested Box Beams in Group D	96
4.5.4	The Ductility of the Tested Box Beams in Group D	97
4.5.4.1	Deflection Ductility	97
4.5.4.2	Curvature Ductility	98
4.5.5	The Strain of the Tested Box Beams in Group D	98
4.5.5.1	Average Strain in Compression Face of Concrete	100
4.5.5.2	Average Strain in Longitudinal Reinforcement	101
4.5.5.3	Average Strain in Shear Reinforcement	102
4.6	Group E	103
4.6.1	The Ultimate Load of the Tested Box Beams in Group E	103
4.6.2	Crack Pattern, Crack Width and Mode of Failure of the Tested Box Beams in Group E	104
4.6.3	The Load-Deflection of the Tested Box Beams in Group E	106
4.6.4	The Ductility of the Tested Box Beams in Group E	107
4.6.4.1	Deflection Ductility	107
4.6.4.2	Curvature Ductility	107
4.6.5	The Strain of the Tested Box Beams in Group E	108
4.6.5.1	Average Strain in Compression Face of Concrete	110
4.6.5.2	Average Strain in Longitudinal Reinforcement	110
4.6.5.3	Average Strain in Shear Reinforcement	111
4.6.5.4	The Strain in Steel plates	112
CHAPTER FIVE		
CONCLUTIONS AND RECOMMENDAIONS		
5.1	Conclusions	114
5.2	Recommendations	116
5.3	Recommendations for Future Studies	116
REFERENCES		117
APPENDIX A		
ABSTRACT (Arabic Version)		

LIST OF FIGURES

Figure No.	Figure Title	Page No.
1-1	Box girder cross sections (Sennah and Kennedy, 2002)	2
1-2	Multicell box girder (Seible and Scordelis, 1994).	4
2-1	The dimensions and reinforcement details of the box beam specimens (Jing, et al., 2015)	8
2-2	The dimensions and reinforcement details of the box girder bridges specimens (Ezeokpube, et al., 2015)	9
2-3	The dimensions and reinforcement details of the beams (Manikandan, et al., 2015)	10
2-4	The dimensions and reinforcement details of the specimens (Waryosh, et al. 2015)	12
2-5	The dimensions and reinforcement details for the hollow control beam (Hemzah, et al., 2020)	14
2-6	dimension and the reinforcement details of the box beam specimens (MA, et al., 2020)	16
2-7	dimension and the reinforcement details of the box girder specimens (Bagherifaez, et al., 2014)	18
2-8	The finite element modeling of (a) two-cell (b) three-cell (c) four-cell rectangular box girder created by using SAP 2000 (Khadiranaikar and Venkateshwar, 2016)	19
2-9	The configuration of the proposed HPCC method: (a) the internal mold and reinforcement; (b) hollow core PC column (Kim, et al., 2017)	21
2-10	The dimensions of the corrugated steel web (Shen, et al., 2018)	22
3-1	Schematic representation of the specimens divisions in the groups	29
3-2	Specimen BRVS (all units are in mm)	32
3-3	Specimen BRIS (all units are in mm)	32
3-4	Specimen B2CVS (all units are in mm)	33
3-5	Specimen B4CVS (all units are in mm)	33
3-6	Specimen BSPVS (all units are in mm)	33
3-7	Specimen BSPIS (all units are in mm)	34
3-8	Details of steel plates of group B (all units are in mm)	34
3-9	Specimen BSPSVC (all units are in mm)	35
3-10	Specimen BSPSHC (all units are in mm)	35
3-11	Details of corrugated steel plates (all units are in mm)	35
3-12	Specimen BCC (all units are in mm)	36
3-13	Specimen BCCSPS (all units are in mm)	36
3-14	Schematic representation of the experimental program for SCC specimens	37
3-15	The slump flow measurements test for SCC material (EFNARC, 2002)	49
3-16	L-box test for SCC (EFNARC, 2002)	50
3-17	Strain gauge locations	59
4-1	First crack loads for Group A	64
4-2	Crack width-load relationship	65
4-3	Central deflection-load relationship	67
4-4	Strain distribution with depth at different load stages for (BRVS)	69
4-5	Strain distribution with depth at different load stages for (B2CVS)	69

4-6	Strain distribution with depth at different load stages for (B4CVS)	70
4-7	Compressive strain-load relationship of Group A	71
4-8	Tensile strain-load relationship of Group A	72
4-9	Strain in shear reinforcement-load relationship of Group A	73
4-10	First crack loads for Group B	75
4-11	Crack width-load relationship of Group B	76
4-12	Central deflection-load relationship of Group B	78
4-13	Strain distribution with depth at different load stages for (BRVS)	80
4-14	Strain distribution with depth at different load stages for (BRIS)	80
4-15	Strain distribution with depth at different load stages for (BSPVS)	81
4-16	Strain distribution with depth at different load stages for (BSPIS)	81
4-17	Compressive strain-load relationship of Group B	82
4-18	Tensile strain-load relationship of Group B	83
4-19	Strain in shear reinforcement-load relationship of Group B	84
4-20	First crack loads for Group C	86
4-21	Crack width-load relationship of Group C	86
4-22	Central deflection-load relationship of Group C	88
4-23	Strain distribution with depth at different load stages for (BRVS)	90
4-24	Strain distribution with depth at different load stages for (BSPSVC)	90
4-25	Strain distribution with depth at different load stages for (BSPSHC)	90
4-26	Compressive strain-load relationship of Group C	91
4-27	Tensile strain-load relationship of Group C	92
4-28	Strain in shear reinforcement-load relationship of Group C	93
4-29	First crack loads for Group D	95
4-30	Crack width-load relationship of Group D	95
4-31	Central deflection-load relationship of Group D	97
4-32	Strain distribution with depth at different load stages for (BRVS)	99
4-33	Strain distribution with depth at different load stages for (BCC)	99
4-34	Strain distribution with depth at different load stages for (BCCSPS)	99
4-35	Compressive strain-load relationship of Group D	100
4-36	Tensile strain-load relationship of Group D	101
4-37	Strain in shear reinforcement-load relationship of Group D	102
4-38	First crack loads for Group E	104
4-39	Crack width-load relationship of Group E	105
4-40	Central deflection-load relationship of Group E	107
4-41	Strain distribution with depth at different load stages for (BSPSVC)	109
4-42	Strain distribution with depth at different load stages for (BSPSHC)	109
4-43	Strain distribution with depth at different load stages for (BCCSPS)	109
4-44	Compressive strain-load relationship of Group E	110
4-45	Tensile strain-load relationship of Group E	111
4-46	Strain in shear reinforcement-load relationship of Group E	112
4-47	Steel plates strain-load relationship (bottom face) of Group E	113
4-48	Steel plates strain-load relationship (side face) of Group E	113
A-1	Stress and strain distribution across box beam depth	A-1
A-2	Dimensions and reinforcement details of the reference box beam	A-1
A-3	Shear force diagram and bending moment diagram for box beam	A-2

LIST OF PLATES

Plate No.	Plate Title	Page No.
2-1	The casting of the beam with circular hollow core (Manikandan, et al., 2015)	11
2-2	The reinforcement steel cages (Waryosh, et al. 2015)	13
2-3	The reinforcement steel cages inside the wooden molds before casting (Hemzah, et al., 2020)	15
2-4	The fabrication of the hollow core PC column: (a) the internal mold; (b) the reinforcement; (c) the precast concrete; (d) the completion (Kim, et al.,2017)	21
2-5	The steel plates used in the work (Ibrahim, et al., 2016)	25
2-6	steel plates with vertical and inclined holes (Khudair and Abbas, 2020)	26
3-1	Al-Sudour graded natural sand	39
3-2	Sieve analysis of fine aggregate	39
3-3	Crushed gravel with 10mm maximum size of aggregate	40
3-4	Sieve analysis of coarse aggregate	40
3-5	Machine used for testing steel bars	43
3-6	Tensile test for steel plates	44
3-7	Manufacturing of shear steel plates	44
3-8	Manufacturing of corrugated steel plates	45
3-9	Manufacturing of steel pipe	45
3-10	Wooden molds used for casting the specimens	46
3-11	Rectangular and circular cells	46
3-12	Mixing process of trial mixes	47
3-13	Slump flow test	49
3-14	L-box test	50
3-15	Steel reinforcement for all specimens	50
3-16	Casting of the specimens	52
3-17	Specimens during the curing stage	52
3-18	White Painting of the specimens	52
3-19	Concrete compressive strength test	53
3-20	Splitting tensile strength test	53
3-21	Modulus of rupture test	54
3-22	Modulus of Elasticity test	55
3-23	LVDT instrument	57
3-24	Micro-Crack meter device	57
3-25	Strain gauge types	58
3-26	Strain gauges installation	60
3-27	Data logger (TDS-530)	60
3-28	Testing machine used to test the box beam specimens	61
4-1	Mode of failure and crack pattern of group A specimens	65
4-2	Mode of failure and crack pattern of group B specimens	76
4-3	Mode of failure and crack pattern of group C specimens	87
4-4	Mode of failure and crack pattern of group D specimens	96
4-5	Mode of failure and crack pattern of group E specimens	105

LIST OF TABLES

Table No.	Table Title	Page No.
2-1	The experimental results (Manikandan, et al., 2015)	11
3-1	Specimens details	30
3-2	Details of box beams names	32
3-3	Physical properties of cement	38
3-4	Chemical composition and main compounds of cement	38
3-5	The grading of fine aggregate	39
3-6	Physical properties of fine aggregate	40
3-7	Grading of coarse aggregate	40
3-8	Physical properties of coarse aggregate	41
3-9	Chemical composition of limestone powder	41
3-10	The properties of superplasticizer	42
3-11	Properties of steel bars	43
3-12	Tensile properties of steel plate	44
3-13	Mixes proportions of self-compacted concrete	47
3-14	Test results of fresh SCC	50
3-15	Experimental results of mechanical properties of concrete	56
4-1	Experimental results for group A	62
4-2	Ductility results for group A	67
4-3	Strain at ultimate load of Group A	69
4-4	Experimental results for group B	74
4-5	Ductility results for group B	78
4-6	Strain at ultimate load of Group B	80
4-7	Experimental results for group C	85
4-8	Ductility results for group C	88
4-9	Strain at ultimate load of Group C	89
4-10	Experimental results for group D	93
4-11	Ductility results for group D	97
4-12	Strain at ultimate load of Group D	98
4-13	Experimental results for group E	103
4-14	Ductility results for group E	107
4-15	Strain at ultimate load of Group E	108

LIST OF SYMBOLS

Symbol	Title
A	Depth of rectangular stress block, mm
A_b	Area of an individual bar, mm ²
A_c	Area of concrete section, mm ²
A_s	Area of main longitudinal tension reinforcement, mm ²
A_v	Area of shear reinforcement, mm ²
bf	Effective flange width, mm
bw	Web width, mm
c	Distance from extreme compression fiber to neutral axis found by equating the internal tension and compression forces, mm
C	Compressive force
d	Effective depth of beam, distance from extreme compression fiber to centroid of longitudinal tension reinforcement, mm
E_c	Modulus of elasticity of concrete, MPa
E_s	Modulus of elasticity of steel reinforcement, MPa
f'_c	150mm*300mm Cylinder compressive strength of concrete, MPa
f_{ct}	Indirect tensile strength (splitting tensile strength), MPa
f_{cu}	Compressive strength of standard cube, MPa
f_r	Modulus of rupture, MPa
f_y	Yield stress, MPa
h	Total depth of box beam
P_{cr}	First crack load, kN
P_u	Ultimate load of box beam, kN
P_y	Yield load of box beam, kN
S	Center-to-center spacing between reinforcement bars, mm
V_c	Nominal shear strength provided by concrete, kN
V_s	Nominal shear strength provided by shear reinforcement, kN
V_n	Nominal shear strength, kN
Δ_u	Deflection at ultimate load
Δ_y	Deflection at yield load
ε_c	Compressive strain in concrete
ε_s	Tensile strain in steel at theoretical moment strength of beam
β₁	Factor relating depth of equivalent rectangular compressive stress block to depth of neutral axis
φ	Diameter of bar, mm
φ_u	Curvature at ultimate
φ_y	Curvature yield

LIST OF TERMINOLOGIES

Terminology	Title
AASHTO	American Association of State Highway and Transportation Officials
ACI	American Concrete Institute
AE	Acoustic Emission
ASTM	American Society for Testing and Materials
B.S	British Standard
c/c	Center to Center of span, mm
CFRP	Carbon Fiber Reinforced Polymer
CFS	Carbon Fiber Sheet
CNC	Computer Numerical Control machine
DC	Double Cells
EFNARC	European Federation of National Trade Associations Representing Concrete
FE	Finite Element
HPCC	Hollow Precast Concrete Columns
I.Q.S	Iraqi Standard Specification
IRC	Indian Road Congress
kN	kilo Newton
mm	Millimeter
MPa	Mega Pascal (N/mm ²)
No.	Number
PC	Precast Column
PVC	Polyvinyl Chloride
RC	Reinforced Concrete
SC	Single Cell
SCC	Self-Compacted Concrete
SP	Superplasticizer
TC	Triple Cells

CHAPTER ONE

INTRODUCTION

1.1 General

Box girder is a commonly used structural member, it consists of two or more webs that are either vertical or inclined, connected with top and bottom flanges to produce single-cell or multi-cell box girder with rectangular or trapezoidal cross-section (**Saxena and Maru, 2013**). As the span increases, the dead load is an important increasing factor, in order to reduce the dead load, the unnecessary materials that do not utilize their full capacity are removed out of the section, which result the shape of the box girder or the cellular structure. The usual reason for choosing the box girder configuration is that the formed closed cell has a very greater torsional stiffness and strength compared to the open section (**Upadhyay and Maru, 2017**).

The concrete box girders are cast in situ or precast in segments (**García-Segura, et al., 2015**). Decks can be reinforced concrete or prestressed concrete or steel. The cross section of the box girder may be in the form of single cell with one box, multiple-spine with separated boxes or multicell with common bottom flange as shown in Figure (1-1) (**Sennah and Kennedy, 2002**).

Box girder bridges are chosen for the span that is ranging from (20m to 40m) for reinforced concrete bridge and (40m to 100m) for prestressed concrete bridges (**Raju, 2016**).

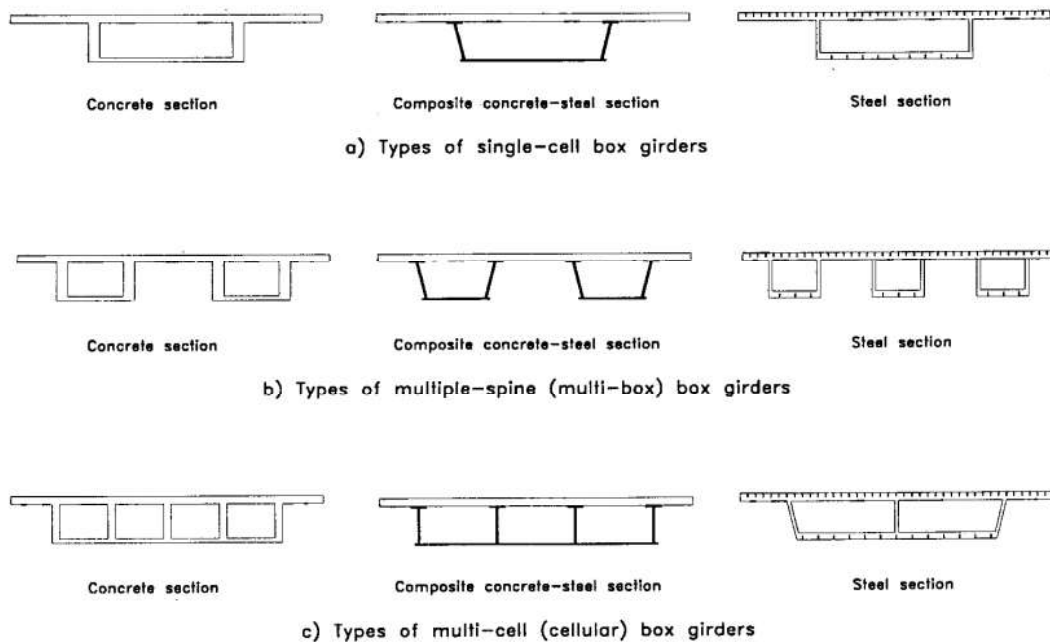


Figure (1-1): Box girder cross sections (Sennah and Kennedy, 2002)

1.1.1 Advantages of Box Girder

1. Hollow sections have been used quite often in the development of bridges, buildings, towers and offshore structures (**Hemzah, et al., 2020**)
2. Hollow cross sections are widely known for being economical, light weight and long span members. (**Waryosh, et al. 2015**)
3. Box concrete sections are commonly used beams, especially for long span bridges to reduce the dead load and save construction and materials cost (**Hemzah, et al., 2020**). And the web can be relatively thin in order to reduce the deadweight (**Lin and Yoda, 2017**).
4. The geometry of box girder has strength to the torsional stresses and to positive and negative bending moments because it has both of top and bottom flanges (**García-Segura, et al., 2015**) and (**Rodriguez., 2004**).
5. Box girder has better load distribution under eccentric load (**Lin and Yoda, 2017**).

6. The large torsional strength and rigidity of the closed section of box girder are favorable to resist the torsional moments due to the curved alignments or the eccentric live load (**Rodriguez., 2004**).
7. The span range for box girder bridge is more compared to T-beam girder bridge, hence, there is less requirement for support points which results in making box girder more economical (**Upadhyay and Maru, 2017**).
8. The box girder section requires less post-tensioning compared to other sections (**Rodriguez., 2004**).
9. Box girder has the ability to carry more load than the I-beam with equal height.
10. The interiors of box girder bridges can be used to accommodate services such as gas pipes, water mains etc.
11. The maintenance of the large box girder is easier in the interior space, it is directly accessible without the use of scaffolding.
12. The alternative space is sealed hermetically and enclosed air may be dried and provide non-corrosive atmosphere.

1.1.2 Disadvantages of Box Girder

1. greater cost associated with fabrication compared with other types of normal beams
2. hard to implement the cold forms
3. risks associated with the working within enclosed spaces

1.1.3 Multi-Cell Box Girder

Reinforced concrete box girders with a multiple cell are widely used. The closed cross-section of the multi-cell makes this structure an ideal one for carrying eccentric loads or torques that are introduced by skew supports. The high internal-statical indeterminacy of this structure allows an excellent transverse distribution for the reactions and the applied load

even without intermediate transverse diaphragms. The reinforced concrete multi-cell box girders are with overload capacities features due to the availability of force redistribution through the structures. Figure (1-2) shows the box girder with three cells (**Seible and Scordelis, 1994**).

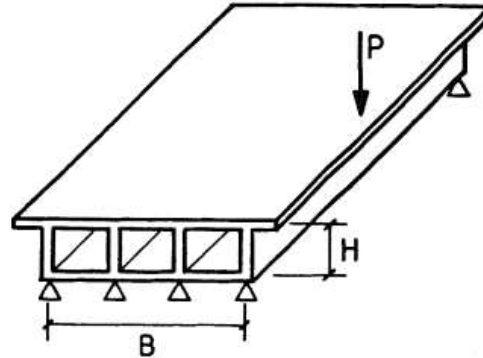


Figure (1-2): Multicell box girder (Seible and Scordelis, 1994)

1.2 Self-Compacted Concrete

Self-compacted concrete (SCC) is the kind of concrete which has the ability to flow freely through the places by its own weight and to fill the areas between the crowded steel bars without the need for vibration (**Kaszynska, 2004**).

Self-compacted concrete has many further names such as flowing concrete (**Bui, et al., 2002**), high-workability concrete and Self-leveling concrete (**Yang, 2004**).

SCC has a great flow capability, can spread and fill the mold without segregation (**ACI Committee 237R-07**).

Self-compacted concrete can be casted in a condition where it is difficult or impossible to use the vibration, for example, underwater concreting, cast in site pile foundations and walls or columns that contain congested steel bars and machine bases (**Patel, et al., 2011**).

1.3 Steel Plates

Some efforts have been made in order to discover a new technique for shear reinforcement including the using of elongated steel plates as shear reinforcement instead of traditional stirrups due to the fast development of the manufacturing by Computer Numerical Control machine (CNC) and some difficulties in the stirrups stand with high cost and time entailed (Ibrahim, et al., 2016).

1.4 Objective of Study

This research was conducted in order to experimentally investigate the structural behavior of reinforced concrete box beams. The main objectives of this study are:

- 1- Studying the effect of using different numbers of cells (one cell, two cells and four cells) in the box beam.
- 2- Studying the effect of using longitudinal shear steel plates contain vertical and inclined rectangular spacings instead of traditional stirrups in the box beam.
- 3- Studying the effect of using vertical and horizontal corrugated steel plates strengthening in the box beam.
- 4- Studying the effect of using circular cell without and with steel plates strengthening instead of rectangular cell in the box beam.

1.5 Study Justification

In recent years, the use of box beam increases significantly, this makes researchers investigate the structural behavior of this type of structural member. Yet, there are no many researches that study the number and the shape of cells in the box beam and the using of steel plates as alternative shear reinforcement as well as the use of steel plates strengthening.

1.6 Research Methodology

The experimental program of this study includes casting and testing ten reinforced concrete box beams and discussing the results in terms of ultimate load, crack pattern, mode of failure, load-deflection relationship, ductility, concrete compressive strain, steel tensile strain, strain in shear reinforcement and strain in steel plates. The experimental work consists of many variables such as:

- Number of cells
- Type of reinforcement
- Type of strengthening
- Shape of cells

1.7 Thesis Layout

The thesis consisted of five chapters:

- **Chapter One** included an introduction to box girder, its advantages and disadvantages, definition of SCC, objective of study, study justification and research methodology.
- **Chapter Two** included a review of previous studies which are related to the present study.
- **Chapter three** explained the experimental program and the properties of the used materials, details of the tested box beams, concrete mix and the test set up.
- **Chapter Four** presented the results of specimens, and the discussion of these results.
- **Chapter Five** viewed the conclusions drawn from this study, in addition to recommendations and suggestions for future research works.